

DESIGN OF LIFTING LUGS

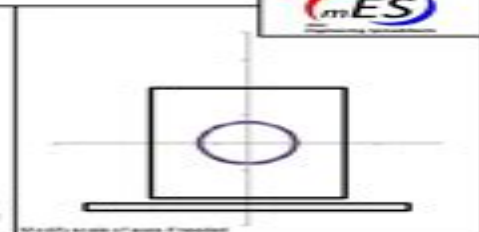
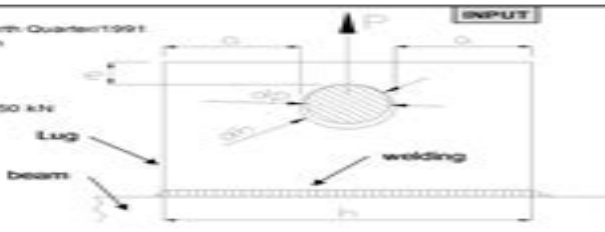
Reference: Design and Construction of Lifting Beams
David T. Ricker, PE, ASCE Engineering Journal, Fourth Quarter 1991
Updated to 2005 ASCE Manual of Steel Construction
ASCE 2005

Date and project ref.:
Designed by: XXXXXX
Company: XXXXXX

Select Units System: S.I.

Unfactored lifting load, $P = 50$ kN
Safety Factor, $S.F. = 1$
Pin diameter, $d_p = 29$ mm
Lug hole diameter, $d_h = 30$ mm
 $a = 15$ mm
 $e = 25$ mm
Lug plate Thickness, $t = 20$ mm
Lug yield stress, $F_y = 235$ MPa
Lug ultimate stress, $F_u = 360$ MPa
Welding length $h = 60,00$ mm

$P = 11,24$ kips.
 $P \cdot S.F. = 11,24$ kips = 50 kN
 $d_p = 1,14$ in.
 $d_h = 1,18$ in.
 $a = 0,59$ in.
 $e = 0,98$ in.
 $t = 0,79$ in.
 $F_y = 34,06$ ksi.
 $F_u = 52,21$ ksi.
 $h = 2,36$ in.



Geometry Checks:
 $d_p < d_h - 1/16"$? No. Pin is considered fitting the hole.
 $a \geq 2t$? No. Increase a or decrease t.
 $a \leq 2t + 63"$? Yes. Use full value of a.
 $a \geq d_h/2$? Yes. Plate proportions are correct.
 $t \geq 1/2"$? Yes. Plate proportions are correct.
 $e \geq 0,67d_h$? Yes. Plate proportions are correct.
 $t \geq 0,25d_h$? Yes. Plate proportions are correct.
 $h \geq 2a + d_h$? Yes. Welding length is correct.

Capacity Checks (capacity values without S.F.):
Failure mode 1: $P_t = 48,56$ kips = 216,0 kN, Tensile Rupture of Lug.
Failure mode 2: $P_p = 27,58$ kips = 122,7 kN, Bearing Capacity of Pin on Hole.
Failure mode 3: $P_s = 21,13$ kips = 94,0 kN, Shear Yielding to Edge of Plate.
Failure mode 4: $P_e = N/A$ kips = N/A, Tension Tear Out to Edge of Plate.

Verification:
Failure mode 1: O.K. $S.F. = 4,32$ $2 \cdot MPA(29+63)/174$ Pg. 151 of Reference, Equation (D5-1) of Manual.
Failure mode 2: O.K. $S.F. = 2,45$ $2 \cdot 77y/10p$ Pg. 152 of Reference, Equation (D7-1) of Manual.
Failure mode 3: O.K. $S.F. = 1,38$ $2 \cdot 77y/10p$ Pg. 152 of Reference, Equation (D3-1) of Manual.
Failure mode 4: N/A. $S.F. = 1,67$ $2 \cdot 77y/10p$ Pg. 152 of Reference, with $P_t = 0,67 F_y$.

Checks of lug bottom section:
 $F_{ax} = h \cdot t \cdot f_y = 282$ kN, Tension resistance of the section.
Verification: O.K. $P \cdot S.F. = F_{ax}$

Welding verification

Material of bottom beam

Yield of bottom beam, $f_y = 355$ MPa
Ultimate stress of bottom beam, $f_u = 555$ MPa



Welding verification (type 1, fillet welding)

Geometry:
Welding length, $h = 60,00$ mm
Number of welding chord, $n_w = 2$
Welding:
Welding throat, $a_w = 5$ mm
Side t_w of weld, $t_w = a_w \cdot \sqrt{2} = 7,07$ mm
Load, $N_{ax} = P \cdot S.F. = 50,00$ kN



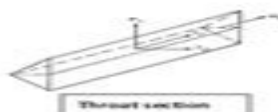
Materials

Yield of the lug, $f_{yL} = 235,00$ MPa
Ultimate stress of lug, $f_{uL} = 360,00$ MPa
Yield of bottom beam, $f_y = 355,00$ MPa
Ultimate stress of bottom beam, $f_u = 510,00$ MPa
Yield of the weakest component, $f_y = f_{yL} = 235,00$ MPa
Ultimate stress weakest component, $f_u = 360,00$ MPa

$\gamma_{M0} = 1,25$
 $\gamma_{M1} = 0,5$
 $\gamma_{M2} = 0,8$
 $\gamma_{M3} = 1$
User value

Stresses on the throat section

Verification as per method of Eurocode 3 EN 1993 1-8:2005
 $\sigma_{t1} = N_{ax} / (n_w \cdot h \cdot a_w) = 58,93$ MPa
 $\sigma_{t2} = N_{ax} / (n_w \cdot h \cdot a_w) = 58,93$ MPa
 $\tau_w = 0$
 $\sigma_{t1} = (\sigma_{t1}^2 + 3 \cdot (\tau_w^2 + \sigma_{t2}^2))^{1/2} = 117,85$ MPa
 $\sigma_{t2} = f_u / (\gamma_{M0} \cdot \gamma_{M1}) = 576,00$ MPa
 $\sigma_{t1}^* = 0,9 \cdot f_u / \gamma_{M0} = 259,20$ MPa



Stresses on the overturned throat section

Verification as per alternative method in EN 1993 1-8:2005
 $\sigma_{t1} = \sigma_{t2} = N_{ax} / (n_w \cdot h \cdot a_w) = 83,33$ MPa
 $\tau_w \cdot \gamma_{M1} = 188,00$ MPa
 $\tau_w \cdot \gamma_{M2} = 235,00$ MPa
 $\sigma_{t1}^* = (\sigma_{t1}^2 + \tau_w^2)^{1/2} = 83,33$ MPa
 $\sigma_{t2}^* = (\sigma_{t2}^2 + \tau_w^2)^{1/2} = 83,33$ MPa



$\sigma_{t1} \leq \sigma_{t1}^*$ OK VERIFIED $S.F. = 4,89$, ratio* = 0,205
 $\sigma_{t2} \leq \sigma_{t2}^*$ OK VERIFIED $S.F. = 4,4$, ratio* = 0,227

$\sqrt{(\sigma_{t1}^2 + \tau_w^2) + \tau_w^2} \leq f_{uL} \cdot \gamma_{M1}$ OK VERIFIED $S.F. = 2,26$, ratio* = 0,443
 $\sqrt{(\sigma_{t2}^2 + \tau_w^2) + \tau_w^2} \leq f_{uL} \cdot \gamma_{M2}$ OK VERIFIED $S.F. = 2,82$, ratio* = 0,355

Simplified method

Tension resistance of the welding:
 $F_{w,t} = f_w \cdot l \cdot (\sqrt{3} \cdot \gamma_{M0} \cdot \gamma_{M1}) = 332,55$ MPa
 $F_{w,t} = a_w \cdot h \cdot f_{w,t} \cdot \gamma_{M0} = 199,53$ kN
 $F_{w,t} \geq N_{ax}$ OK VERIFIED $S.F. = 3,99$, ratio* = 0,251

(*) additional safety factor with respect to the S.F. already considered above in the load (N_{ax})

Welding verification (type 2, full penetration)

Eurocode 3 EN 1993 1-8:2005
Joint resistance is equal to the weakest material component

Data:
Lug plate thickness, $t = 20,00$ (mm)
Welding length, $h = 60,00$ (mm)
dimension c ($c \leq 3$ mm) $c = 3$ (mm)
Known values:
Welding side, $t1 = 8$ (mm)
Welding height, $h1 = 10$ (mm)
Welding throat, $a1 = 12,81$ (mm)
Welding angle 1, $\alpha1 = 51,34$ (deg)
Welding side, $t2 = 9,00$ (mm)
Welding height, $h2 = 10$ (mm)
Welding throat, $a2 = 13,45$ (mm)
Welding angle 2, $\alpha2 = 48,01$ (deg)

$a1 + a2 \geq t$ OK VERIFIED

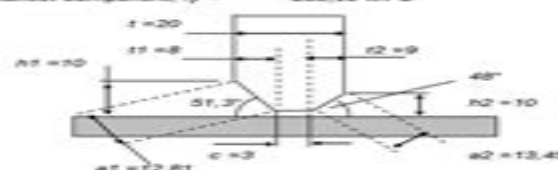
Tension resistance of the welding:
 $F_{w,t} = h \cdot t \cdot f_y = 282,00$ kN
 $F_{w,t} \geq N_{ax}$ OK VERIFIED $S.F. = 5,64$, ratio* = 0,177

Materials

Yield of the lug, $f_{yL} = 235,00$ MPa
Yield of bottom beam, $f_y = 355,00$ MPa
Yield of the weakest component, $f_y = 235,00$ MPa

Load

Load, $N_{ax} = P \cdot S.F. = 50,00$ kN



(*) additional safety factor with respect to the S.F. already considered above in the load (N_{ax})

Lifting Lug Design Manual

Daniela Niemeyer



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