

1.2 Do problem 1.1 with $M = \alpha T$ where $\alpha = 0$ to ∞ .

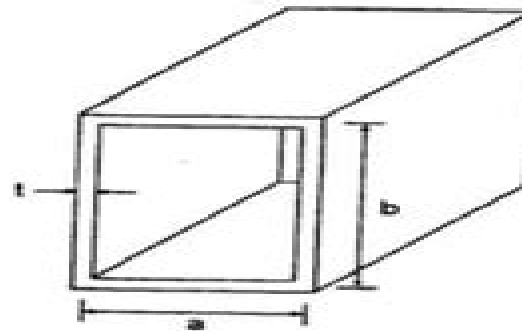


Figure 1.16 Closed thin-walled section

Solution:

(1) The bending stress of beams is $\sigma = \frac{My}{I}$, where y is the distance from the neutral axis. The moment of inertia I of the cross-section can be calculated by considering the four segments of thin walls and using the formula for a rectangular section with height h and width w , $I = \sum (\frac{I}{12} wh^3 + Ad^2)$ in which A is the cross-sectional area of the segment and d is the distance of the centroid of the segment to the neutral axis. Note that the Parallel Axis Theorem is applied. The result is $I = 2 \cdot \frac{1}{12} tb^3 + 2 \cdot [\frac{1}{12} \cdot at^3 + (at) \cdot (\frac{b}{2})^2] \approx \frac{tb^2}{6} (3a + b)$, assuming that t is very small.

(2) The shear stress due to torsion for a closed thin-walled section shown above is

$$\tau = \frac{T}{2abt}.$$

(3) Two approaches are employed to find the solution.

(i) Assume that the bending stress reaches the allowable $\sigma_{allowable}$ first and find the corresponding bending maximum bending moment. Then apply the stated loading condition of $M = \alpha T$ to check whether the corresponding τ_{max} has exceeded the allowable shear stress $\tau_{allowable}$. If this condition is violated, then the optimized b/a ratio is not valid.

$$(a) \quad \sigma \Big|_{r=\frac{b}{2}} = \frac{My}{I} = \frac{M \cdot \frac{b}{2}}{\frac{tb^2}{6} (3a + b)} = \frac{3M}{tb(3a + b)}$$

When given $I = 2(a + b)$ as a constant, a can be expressed in terms of b

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