

1.2 Do problem 1.1 with  $M = \alpha T$  where  $\alpha = 0$  to  $\infty$ .

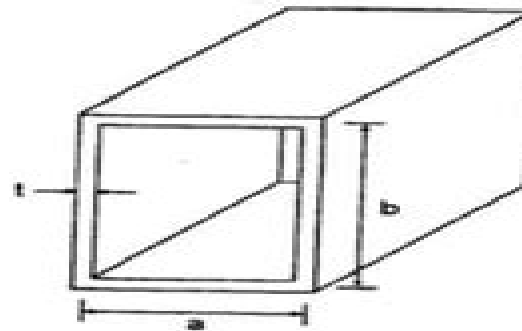


Figure 1.16 Closed thin-walled section

**Solution:**

(1) The bending stress of beams is  $\sigma = \frac{My}{I}$ , where  $y$  is the distance from the neutral axis. The moment of inertia  $I$  of the cross-section can be calculated by considering the four segments of thin walls and using the formula for a rectangular section with height  $h$  and width  $w$ ,  $I = \sum (\frac{I}{12} wh^3 + Ad^2)$  in which  $A$  is the cross-sectional area of the segment and  $d$  is the distance of the centroid of the segment to the neutral axis. Note that the Parallel Axis Theorem is applied. The result is  $I = 2 \cdot \frac{1}{12} tb^3 + 2 \cdot [\frac{1}{12} \cdot at^3 + (at) \cdot (\frac{b}{2})^2] \approx \frac{tb^2}{6} (3a + b)$ , assuming that  $t$  is very small.

(2) The shear stress due to torsion for a closed thin-walled section shown above is

$$\tau = \frac{T}{2abt}.$$

(3) Two approaches are employed to find the solution.

(i) Assume that the bending stress reaches the allowable  $\sigma_{allowable}$  first and find the corresponding bending maximum bending moment. Then apply the stated loading condition of  $M = \alpha T$  to check whether the corresponding  $\tau_{max}$  has exceeded the allowable shear stress  $\tau_{allowable}$ . If this condition is violated, then the optimized  $b/a$  ratio is not valid.

$$(a) \quad \sigma \Big|_{r=\frac{b}{2}} = \frac{My}{I} = \frac{M \cdot \frac{b}{2}}{\frac{tb^2}{6} (3a + b)} = \frac{3M}{tb(3a + b)}$$

When given  $I = 2(a + b)$  as a constant,  $a$  can be expressed in terms of  $b$

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## **Table of Contents Mechanics Of Aircraft Structures Solutions**

1. Understanding the eBook Mechanics Of Aircraft Structures Solutions
  - The Rise of Digital Reading Mechanics Of Aircraft Structures Solutions
  - Advantages of eBooks Over Traditional Books
2. Identifying Mechanics Of Aircraft Structures Solutions
  - Exploring Different Genres
  - Considering Fiction vs. Non-Fiction
  - Determining Your Reading Goals
3. Choosing the Right eBook Platform
  - Popular eBook Platforms
  - Features to Look for in an Mechanics Of Aircraft Structures Solutions
  - User-Friendly Interface
4. Exploring eBook Recommendations from Mechanics Of Aircraft Structures Solutions
  - Personalized Recommendations
  - Mechanics Of Aircraft Structures Solutions User Reviews and Ratings
  - Mechanics Of Aircraft Structures Solutions and Bestseller Lists
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  - Mechanics Of Aircraft Structures Solutions Enhanced eBook Features
- 7. Enhancing Your Reading Experience
  - Adjustable Fonts and Text Sizes of Mechanics Of Aircraft Structures Solutions
  - Highlighting and Note-Taking Mechanics Of Aircraft Structures Solutions
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- 8. Staying Engaged with Mechanics Of Aircraft Structures Solutions
  - Joining Online Reading Communities
  - Participating in Virtual Book Clubs
  - Following Authors and Publishers Mechanics Of Aircraft Structures Solutions
- 9. Balancing eBooks and Physical Books Mechanics Of Aircraft Structures Solutions
  - Benefits of a Digital Library
  - Creating a Diverse Reading Collection Mechanics Of Aircraft Structures Solutions
- 10. Overcoming Reading Challenges
  - Dealing with Digital Eye Strain
  - Minimizing Distractions
  - Managing Screen Time
- 11. Cultivating a Reading Routine Mechanics Of Aircraft Structures Solutions
  - Setting Reading Goals Mechanics Of Aircraft Structures Solutions
  - Carving Out Dedicated Reading Time
- 12. Sourcing Reliable Information of Mechanics Of Aircraft Structures Solutions
  - Fact-Checking eBook Content of Mechanics Of Aircraft Structures Solutions
  - Distinguishing Credible Sources
- 13. Promoting Lifelong Learning
  - Utilizing eBooks for Skill Development

- Exploring Educational eBooks

### 14. Embracing eBook Trends

- Integration of Multimedia Elements
- Interactive and Gamified eBooks

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